



1 Article

# 2 Effects of key properties of rainfall series on

# hydrologic design of sustainable urban drainage systems (SUDS)

# 5 Rodrigo Jodra-Lopez<sup>1,\*</sup>, Alvaro Sordo-Ward<sup>1</sup>, Ivan Gabriel-Martin<sup>1</sup> and Luis Garrote<sup>1</sup>

## 6 Received: date; Accepted: date; Published: date

7 Academic Editor: name

 8
 <sup>1</sup> Department of Civil Engineering: Hydraulics, Energy and Environment, Universidad Politécnica de Madrid, Madrid, Spain; alvaro.sordo.ward@upm.es (A.S.-W); i.gabriel@upm.es (I.G); l.garrote@upm.es

- 10 (L.G.)
- 11 \* Correspondence: rs.jodra@alumnos.upm.es (R.J.); Tel.: +34-913-336-6672

12 Abstract: The aims of this study are to quantify the effects of key properties of rainfall time series 13 on the hydrologic design of sustainable urban drainage systems (SUDS), to test a method for their 14 estimation from daily time series and to quantify their uncertainty. Several typologies of SUDS 15 infrastructures are designed to achieve a target treatment capacity. This target capacity is usually 16 defined according to two methods: treating a percentage of the total volume of rainfall (50, 80, 90, 17 95, 99%) or treating a percentage of the total number of rainfall events (50, 80, 90, 95, 99%). We 18 considered the city of Madrid as the case study, compiling 58 years of observed data (10-minute 19 time step) and aggregating to daily time series. We obtained the design parameters from the full 20 resolution dataset and for different storm thresholds (0, 1 and 2 millimetres). Second, we determined 21 the design parameters from the aggregated daily time series by applying a temporal stochastic 22 rainfall generator model (RainSimV3). Finally, we estimated the model parameters from daily data 23 and generated 100 series of 58 years at 10-minute time step, and compared the results. Results 24 showed a good agreement compared to the 10-minute time step rainfall series. The different 25 thresholds selected do not affect in a relevant way the calculation by percentage of the total volume, 26 in the case of calculation by events, the threshold can vary the design volume up to 30%. Further 27 research includes the analysis of different climate locations.

Keywords: SUDS; sutainable drainage systems; hydrologic design; stochastic rainfall generator;
 stochastic approach.

- 30 PACS: J0101
- 31

## 32 **1. Introduction**

Currently, more than half of the world population lives in urban areas and a growth is expected [1]. Human activity on the basins induced changes on the hydrological characteristics and higher costs for the construction and maintenance of conventional drainage systems [2,3]. The design and implementation of sustainable urban drainage systems (SUDS) could contribute to mitigate this problem, by reducing the runoff volume, the peak flow, as well as reducing outlet contaminants [4,5,6].

The design of urban drainage systems has traditionally been carried out from historical data or through design storms [7]. However, the small size of the urban watersheds and short response time, make it necessary to consider the rainfall series at a sub-hourly time-step [8,9]. From a global perspective, daily time-step rainfall data is the most common available information. Different downscaling methodologies have been widely studied for urban applications [10,11] but their application to SUDS design was not fully developed [12-16]. In this study, the temporal disaggregation was analysed from daily data to 10-minute resolutions, based on the Neyman-Scott Rectangular Pulse Method in a single-site and by using the RainSim V.3 model [16]. The aims of this study are to quantify the effects of key properties of rainfall time series on the hydrologic design of sustainable urban drainage systems (SUDS), to test a method for their estimation from daily time series and to quantify their uncertainty.

#### 50 2. Materials and Methods

51 We analysed the effect of the rainfall design on two types of parameters commonly used to 52 design SUDS: 1) those that treat a percentage of the total volume of accumulated rainfall series (50, 53 80, 90, 95, 99%, and named as V50, V80, V90, V95, V99) and, 2) those that treat a percentage of the 54 total number of rainfall events (50, 80, 90, 95, 99%, and named as N50, N80, N90, N95, N99) during 55 the analysed rainfall series. The methodology applied was based on the stochastic generation of 10-56 minute time-step rainfall series (using the RainSimV3 model). First, we obtained the SUDS design 57 parameters from the observed 10-minute rainfall series (58 years) and from aggregated daily rainfall 58 time series. We estimated the parameters of the RainSimV3 model from the observed daily time 59 series. Third, we generated 100 series of 58 years at 10-minute time step. Fourth, we validated the 60 Rain Sim V3 model by comparing the intensity-duration-frequency curves (IDF) and the rainfall 61 frequency curves obtained from observed and simulated time series. Fifth, we calculated the SUDS 62 parameters and, finally, we compared and analysed the results.

#### 63 2.1. *Case study*

We considered the city of Madrid as the case study. We compiled 58 years of observed data (10minute time step, from 1941 To 1998) from the Madrid Retiro gauge station (id station: 3195). Figure shows the gauge location, it is centred on the city and located at an altitude of 667 m.a.s.l. (referred to the Alicante sea level). Madrid has a semi-arid climate with an average annual precipitation of 441mm. The pluviography measurement series has a minimum appreciation of 0.2 mm. By aggregation, the daily data has been obtained.

70



- 71
- 72 73



#### 74 2.2. Stochastic rainfall generation

75 The methodology applied was based on the stochastic generation of 10-minute time-step rainfall 76 series by using the RainSimV3 model. First, we estimated the parameters of the RainSimV3 model 77 from the observed daily time series. Second, we generated 100 series of 58 years at 10-minute time 78 step. Third, we validated the model by comparing: a) the intensity-duration-frequency curves (IDF)

79 obtained by the model and from observed data and from previous studies [17-21], and b) the rainfall

80 frequency curves obtained from observed and simulated time series and by accounting different

81 storm durations (10 minutes, 1 h and 24 h.)

#### 82 2.3. Estimation of design parameters (SUDS)

83 To estimate the SUDS parameters, we first identified the storms from the rainfall series. As, 84 usually the available rainfall series are daily time-step and the minimum inter-storm period 85 considered is a day [22], we assumed independent storms those with a minimum inter-storm period 86 of 24 h. We obtained the SUDS design parameters from the observed 10-minute rainfall series (58 87 years) and from the aggregated daily rainfall time series. We calculated the design parameters for 88 different storm thresholds (0, 1 and 2 millimetres), that is, by considering all identified storms 89 (threshold = 0 mm), by considering the storms with total depth higher than 1 mm., and 2 mm. 90 respectively. For each storm threshold, we generated 100 series of 58 years at 10-minute time step 91 and calculated the SUDS parameters. Finally, we compared and analysed the results.

#### 92 3. Results and discussion

#### 93 3.1. Stochastic rainfall validation

94 Table 1 shows the values of the IDF curves obtained from the observed 10-minute time-series, 95 the simulated series and previous studies. Casas-Castillo et al. [17] used 5-minute rainfall series from 96 1940 to 2012. AEMET [18] obtained the IDF curves by fitting the observed data (10-minute series from 97 1942 to 2002) to a SQRT-ETmax. Distribution function [23]. Finally, results from the application of the 98 national 5.2IC [19] are shown (rainfall values were extracted from the MAXPLU study [20]. Results 99 show that the median values from the stochastic simulations (with parameters adjusted using 100 observed daily data) have a good agreement compared with the results from the 10-minute data, with 101 differences smaller than 10 % for most of the analysed storm durations and return periods. Moreover, 102

the differences are within the 95 % confidence interval estimated by the stochastic simulations.

103

104 105

Table 1. Comparison of IDF curves according to different sources of data and the methods applied. Dur correspond to the storm duration in minutes and Tr the return period in years. Values are presented in mm/h.

	Ob	served	IDF cui	rves	Sin	nulated	IDF cu	rves		AEME	Г (2003)		Casas	-Castill	lo et al.	(2016)	5.2-I	C Retire	o (MAX	PLU)
Dur/Tr	2	5	10	15	2	5	10	15	2	5	10	15	2	5	10	15	2	5	10	15
10	35.6	49.8	59.6	65.5	37.8	55.2	67.8	74.4	34.0	52.0	65.0	71.3	38.6	55.5	68.3	75.7	35.7	47.6	55.2	59.6
20	23.7	37.0	45.1	49.3	27.3	40.5	50.7	56.4	26.0	38.0	48.0	52.3	25.5	36.6	45.0	49.9	25.2	33.6	38.9	42.0
30	19.1	27.7	37.3	38.2	20.6	30.0	37.2	41.8	20.0	30.0	38.0	41.7	19.6	28.1	34.6	38.4	20.3	27.1	31.4	33.8
60	11.6	17.7	20.3	22.0	12.6	17.8	21.5	23.9	12.5	18.0	22.2	24.1	12.2	17.5	21.6	23.9	13.8	18.3	21.3	22.9
120	7.5	10.7	12.8	16.1	7.1	10.0	12.2	13.5	7.9	10.9	13.0	14.0	7.5	10.8	13.2	14.7	9.1	12.1	14.0	15.1
360	3.8	4.8	5.3	6.0	3.3	4.4	5.1	5.5	3.8	5.0	5.9	6.3	3.4	4.9	6.0	6.7	4.4	5.8	6.8	7.3
720	2.3	3.0	3.3	3.5	2.2	2.8	3.3	3.6	2.4	3.1	3.6	3.8	2.1	3.0	3.6	4.0	2.7	3.5	4.1	4.4
1440	1.4	1.8	2.2	2.3	1.4	1.8	2.1	2.3	1.5	1.9	2.2	2.3	1.2	1.8	2.2	2.4	1.6	2.1	2.4	2.6

106

	Observed IDF cu			rves	Simulated ID			rves		AEMET (2003)			Casas-Castillo et al. (2016)				5.2-IC Retiro (MAXPLU			(PLU)	
Dur/Tr		2	5	10	15	2	5	10	15	2	5	10	15	2	5	10	15	2	5	10	15
10						6.1	10.9	13.7	13.5	-4.6	4.5	9.0	8.8	8.3	11.5	14.5	15.5	0.2	-4.4	-7.4	-9.1
20						15.3	9.4	12.3	14.3	9.8	2.7	6.3	6.0	7.7	-1.1	-0.3	1.1	6.4	-9.2	-13.8	-14.9
30		Er	rror	[%]		8.0	8.4	-0.4	9.3	4.9	8.4	1.7	9.0	2.8	1.6	-7.4	0.4	6.5	-2.0	-15.9	-11.6
60						8.9	0.6	5.9	8.4	8.0	1.8	9.3	9.3	5.4	-1.1	6.4	8.4	19.3	3.5	4.9	3.9
120	100	. <u>(Com</u>		Observe erved	d)	-4.9	-6.5	-4.4	-16.7	5.8	2.0	1.9	-13.3	0.5	1.0	3.4	-8.9	21.9	13.2	9.7	-6.4
360						-12.6	-9.7	-3.1	-8.8	-0.3	3.3	12.1	4.5	-10.8	1.3	14.0	11.1	15.4	19.9	29.2	21.0
720						-4.8	-5.6	-0.6	2.3	4.7	3.3	7.6	8.0	-8.4	0.0	7.6	13.6	17.8	16.6	22.5	25.0
1440						0.0	0.0	-4.2	0.0	7.1	5.6	0.6	0.0	-14.3	0.0	2.2	4.3	14.3	16.7	9.7	13.0

107 108

109

Figure 2 shows the rainfall frequency curves (RFC) corresponding to rainfall durations of 10 minutes, 1 h and 24 h respectively. Simulated RFC curves for 1 h and 24 h show an excellent Journal Name 2018, x, x

110agreement compared with their correspondent observed RFC curves (calculated from 10-minute111time series). It should be noted that the simulated RFC curves were generated with parameters112estimated from daily data. Thus, the proposed stochastic procedure has a good predictive113capacity for extreme value estimation.

114



115

116Figure 2. Comparison of the estimated rainfall frequency curves corresponding to 10-minutes storm117duration (D, red dots), 1 h and 1 day, their corresponding stochastic simulation (red line) and 95 %118confidence bound (cyan area).

119 3.2. Design parameters analysis (SUDS)

### 120 3.2.1. Parameters based on rainfall volume

121 Figure 3 and Table 2 shows, for the different storm thresholds considered, the values and 122 uncertainty of the V50, V80, V80, V90, V90, V95 and V99, derived from the 100 analysed series. In 123 addition, values obtained from the observed 10-minute and daily time series are also plotted. 124 Although, results show better performance of the stochastic series assuming a threshold value of 2 125 mm compared with 1 mm and considering all events, the SUDS design parameters, for this case 126 study, did not present high sensitivity. For V50, V80 and V90, better results were obtained for the 127 stochastic approach than for daily data. Thus, starting from observed daily data the stochastic 128 approach could obtain similar SUDS design values than using observed daily data but also estimates 129 a 10-minute time-step series, very useful for SUDS design; for example, for a better estimation of 130 storm characteristics as temporal distribution of storms, time among events, maximum and mean 131 rainfall intensities, among others. Finally, results from observed 10-minute time step are within the 132 95 % confidence bound of the stochastic simulation.

133



134Figure 3. Comparison of the SUDS parameters based on rainfall event volumes. Red line represents135the event rainfall volume value that ensure a treatment of the 50, 80, 90, 95 and 99 % of the136accumulated rainfall depth within the analysed period (58 years) at 10-minute time step. Green line137corresponds to daily time step, blue lines correspond to the stochastic simulations at 10-minute time138step and yellow dotted line represent the median values of the stochastic simulation.

139

140 141

**Table 2.** Comparison of the SUDS design values (V50, V80, V90, V95 and V99) in mm. by using the10-minute observed data, daily observed data and stochastic generated data, and by consideringdifferent storm thresholds (0, 1 and 2 mm).

	Observed 10	Observed		Simula	ited	Error Daily	Error Simulated		
	min	daily	Min	Max	Median	%	%		
			Thres	hold 0m	ım				
V50	7.35	8.57	7.23	8.25	7.8	17%	6%		
V80	19.53	22.89	16.4	20.3	18	17%	-8%		
V90	29.39	33.64	23.4	30.8	26.15	14%	-11%		
V95	40.17	44.81	30	42.9	34.5	12%	-14%		
V99	65.38	69.94	44.7	112.5	57.25	7%	-12%		
			Thres	hold 1m	ım				
V50	7.49	8.72	7.29	8.32	7.85	16%	5%		
<b>V80</b>	19.78	22.96	16.5	20.4	18	16%	-9%		
V90	29.6	33.93	23.5	30.9	26.2	15%	-11%		
V95	40.15	44.8	30.4	42.9	34.6	12%	-14%		
V99	67.55	69.93	44.7	112.5	57.25	4%	-15%		

Journal Name 2018, x, x

	Threshold 2mm											
V50	7.75	8.95	7.41	8.41	7.96	15%	3%					
<b>V80</b>	20	23.3	16.6	20.5	18.2	17%	-9%					
<b>V90</b>	29.9	34.21	23.6	31.2	26.25	14%	-12%					
V95	40.82	45	30.4	42.9	34.65	10%	-15%					
V99	67.55	69.94	44.7	112.5	57.25	4%	-15%					

142

143 3.2.2. Design parameters based on number of events

144 Figure 4 and Table 3 show, for the different storm thresholds considered, the values and 145 uncertainty of the N50, N80, N90, N95 and N99, derived from the 100 analysed series. In addition, 146 values obtained from the observed 10-minute and daily time series are also presented. Results show 147 a good performance of the simulated N80, N90, N95 and N99 and by considering a storm threshold 148 of 2 mm. For design parameters based on the number of identified events, both the storm threshold 149 considered and the criteria adopted to identify independent storms affect significantly to the results. 150 For N80, N90, N95 and N99, better results were obtained for the stochastic approach than for 151 daily data (storm threshold = 2 mm).

152



153 154 155

156

157

**Figure 4.** Comparison of the SUDS parameters based on the number of rainfall events. Red line represents the event rainfall volume value that ensure a treatment of the 50, 80, 90, 95 and 99 % of the total storm events within the analysed period (58 years) at 10-minute time step. Green line corresponds to daily time step, blue lines correspond to the stochastic simulations at 10-minute time step and yellow dotted line represent the median values of the stochastic simulation.

158 159

160

**Table 3.** Comparison of the SUDS design values (N50, N80, N90, N95 and N99) in mm. by using the 10-minute observed data, daily observed data and stochastic generated data, and by considering different storm thresholds (0, 1 and 2 mm).

	Observed 10	10 Observed		Simulat	ted	Error Daily	Error Simulated		
	min	daily	Min	Max	Median	%	%		
			Thres	hold 0mi	m				
N50	4.2	4.75	7.2	8.8	8	13%	90%		
N80	13.03	15.42	16.4	19	17.9	18%	37%		
N90	22.15	26.1	23.13	27.32	25.35	18%	14%		
N95	31.29	37	29.9	36.48	33.11	18%	6%		
N99	55.02	62.83	46.19	63.19	52.4	14%	-5%		
			Thres	hold 1mi	n				
N50	6.54	6.85	8.5	10.3	9.2	5%	41%		
N80	16.09	18.15	17.5	20.02	19.07	13%	19%		
N90	25.07	29.63	24.13	28.88	26.4	18%	5%		
N95	34.12	40.9	30.55	38.07	34.3	20%	1%		
N99	59.015	67.67	47.91	66.029	53.6	15%	-9%		
			Thres	hold 2mi	m				
N50	8	8.42	9.5	11.2	10.2	5%	28%		
N80	18.18	20.84	18.5	21	20	15%	10%		
N90	27.82	31.77	25.3	30.3	27.45	14%	-1%		
N95	36.8	43.05	31.5	39.2	35.45	17%	-4%		
N99	62.84	68.6	48.3	69.95	57.76	9%	-8%		

#### 161 5. Conclusions

162 The use of a stochastic approach for the generation of 10-minute time step rainfall series from 163 daily observed data showed a good agreement compared to the 10-minute time step rainfall series. 164 The proposed approach allows the estimation of very useful rainfall characteristics for SUDS design 165 as the temporal distribution of storms, time among events, maximum and mean storm rainfall 166 intensities, among others. For the case study analysed, the stochastic approach generates 10-minute 167 rainfall series with IDF curves and rainfall frequency curves similar to observed data.

Parameters to design SUDS based on the number of storm identified have more dependence to the criteria adopted to define independent storms or the minimum value of rainfall to consider a storm. However, the parameter to design SUDS based on the volume of the storm events are not sensible to the mentioned criteria.

This approach allows to quantify the associated uncertainty of the values adopted to the design of SUDS. It should be noted that we applied this methodology to one location. This might limit the generalization of the results obtained. Further research will be focused on the application of this approach on locations with different climate characteristics.

Acknowledgments: We acknowledge the financial support of the "Programa propio: ayudas a proyectos de I+D de investigadores posdoctorales" of the Universidad Politécnica de Madrid and the AEMET support for providing the rainfall data. The authors also acknowledge the funds from the Universidad Politécnica de Madrid in the framework of their Program "Ayudas para contratos predoctorales para la realización del doctorado en sus escuelas, facultad, centro e institutos de I+D+i" and the funds from Carlos Gonzalez Cruz Foundation.

181 Author Contributions: Rodrigo Jodra-López led the numerical calculations, led the analysis and discussion of 182 results and participated in the paper writing. Alvaro Sordo-Ward led the design of the proposed methodology, 183 participated in the analysis and discussion of results and led the paper writing. Ivan Gabriel-Martin participated 184 in the numerical calculations, the analysis and discussion of results and participated in the paper writing. Luis 185 Carrete contributed to the numerical calculations of the proposed methodology of the proposed methodology.

185 Garrote contributed to the general idea of the research, participated in the analysis and discussion of the results.

- 187 of the study; in the collection, analyses, or interpretation of data; in the writing of the manuscript, and in the
- 188 decision to publish the results.

#### 189 References

- United Nations. World urbanization prospects: the 2011 revision. NewYork: United Nations Department of Economic and Social Affairs(Population Division); 2011.
- Hair, L.; Clements, J.; Pratt, J.: Insights on the Economics of Green Infrastructure: A Case Study Approach.
   Water Environment Federatio. WEFTEC 2014: Session 310 through Session 317, pp. 5556-5585(30).
   10.2175/193864714815938869.
- Sordo-Ward A.; Bianucci, P.; Garrote L., Granados, A.; How safe is hydrologic infrastructure design?
  Analysis of factors affecting extreme flood estimation. *J.Hydrol. Eng.* 2014, 19, 12, doi: 10.1061/(ASCE)HE.1943-5584.0000981.
- Arora, A.S.; Reddy, A.S. Multivariate analysis for assessing the quality of stormwater from different urban surfaces of the Patiala City, Punjab (India). *Urban Water J.* 2013, 10, 422–433. 10.1080/1573062X.2012.739629.
- Hatt, B.E.; Fletcher, T.D.; Walsh, C.J.; Taylor, S.L. The influence of urban density and drainage
   infrastructure on the concentrations and loads of pollutants in small streams. *Environ. Manag.* 2004, 34, 112–
   124. 10.1007/s00267-004-0221-8.
- Woods-Ballard, B.; Wilson, S.; Udale-Clarke, H-;Illman,S.; Scott, T.; Ashley, R.; Kellagher, R.; 2015. The
  SUDS Manual, C753. Ciria, London, UK.
- Mikkelsen, P.S. Madsen, H. K.Arnbjerg-Nielsen, K. Jørgensen, H.K. Rosbjerg, D. Harremoës, P. A rationale
   for using local and regional point rainfall data for design and analysis of urban storm drainage systems.
   *Water Science and Technology* 1998. 37, 7-14. 10.1016/S0273-1223(98)00310-2.
- Bennett, N.D., Croke, B.F.W., Guariso, G., Guillaume, J.H.A., Hamilton, S.H., Jakeman, A.J., Marsili-Libelli,
   S., Newham, L.T.H., Norton, J.P., Perrin, C., Pierce, S.A., Robson, B., Seppelt, R., Voinov, A.A., Fath, B.D.,
   Andreassian, V.. Characterising performance of environmental models. *Environ. Model. Soft.* 2013 40, 1e20.
   http://dx.doi.org/10.1016/j.envsoft.2012.09.011.
- 212 9. Cowpertwait, PSP. A spatial-temporal point process model of rainfall for the Thames catchment, UK. J.
   213 Hydrol. 2006, 330, 586-595. 10.1016/j.jhydrol.2006.04.043.
- 214
   10. Rodriguez-iturbe, I.; Depower, B.F.; Valdes, J.B., J Geophys Res Atmos. 1987, 92, 9645-9656, 10.1029/JD092iD08p09645.
- Entekhabi, D; Rodriguez-Iturbe, I; Eagleson, Ps. Probabilistic representation of the temporal rainfall process
   by a modified Neyman-Scott rectangular pulses model parameter-estimation and validation. *Water Resour Res* 1989, 25, 295-302, 10.1029/WR025i002p00295.
- 219
  12. Paschalis, A; Fatichi, S; Molnar, P; Rimkus, S; Burlando, P. On the effects of small scale space-time variability of rainfall on basin flood response. J. Hydrol. 2014, *514*, 313-327, 10.1016/j.jhydrol.2014.04.014.
- 13. Kaczmarska, J; Isham, V; Onof, C. Point process models for fine-resolution rainfall. *HYDROLOG SCI J.* 2014, 59, 1972-1991, 10.1080/02626667.2014.925558.
- 14. Khaliq, MN; Cunnane, C. Modelling point rainfall occurrences with the Modified Bartlett-Lewis
   Rectangular Pulses Model. *J. Hydrol.* 1996, *180*, 109-138, 10.1016/0022-1694(95)02894-3.
- 225 Burton, A ; Kilsby, CG; Fowler, HJ; Cowpertwait, PSP; O'Connell, PE. RainSim: A spatial-temporal 15. 226 stochastic rainfall modelling system. Environ Modell Softw. 2008, 23. 1356-1369, 227 10.1016/j.envsoft.2008.04.003.
- 16. Cowpertwait, PSP. A spatial-temporal point process model with a continuous distribution of storm types.
   Water Resour Res. 2010, 46, 10.1029/2010WR009728.
- 17. Casas-Castillo, MC.; Rodriguez-Sola, R.; Navarro, X.; Russo, B.; Lastra, A.; Gonzalez, P.; Redano, A. On the
   consideration of scaling properties of extreme rainfall in Madrid (Spain) for developing a generalized
   intensity-duration-frequency equation and assessing probable maximum precipitation estimates. *Theor Appl Climatol.* 2018, 131, 573-580, 10.1007/s00704-016-1998-0.
- 18. INM (AEMET), 2003. Curvas de intensidad–duración–frecuencia. [CD, Recurso Electrónico]. Ministerio de
   Medio Ambiente, Madrid, España.
- 236 19. Ministerio de Fomento. *Instrucción de Carreteras 5.2-IC Drenaje Superficial;* Dirección General de Carreteras;
   237 Ministerio de Fomento: Madrid, Spain, 2016. (In Spanish)

- 238 20. Ministerio de Fomento. Máximas Lluvias Diarias en la España Peninsular; Dirección General de Carreteras; 239 Ministerio de Fomento: Madrid, Spain, 1999. (In Spanish)
- 240 21. Sordo-Ward A.; Bianucci P.; Garrote L.; Granados A. The influence of the annual number of storms on the 241 derivation of the flood frequency curve through event-based simulation. Water. 2016, 8, 335, 242 doi:10.3390/w8080335.
- 243 22. Stormwater Management Manual. City of Portland. 2016. Available online: 244 https://www.portlandoregon.gov/bes/64040
- 245 23. Etoh, T.; Murota, A.; Nakanishi, M. SQRT-Exponential Type Distribution of Maximum. In Proceedings of
- 246 the International Symposium on Flood Frequency and Risk Analyses, Louisiana State University, Baton
- 247 Rouge, LA, USA, 14-17 May 1986; Shing, V.P., Ed.; Reidel Publishing Company: Boston, MA, USA, 1987; 248



pp. 253-264.

© 2018 by the authors; licensee MDPI, Basel, Switzerland. This article is an open access article distributed under the terms and conditions of the Creative Commons by Attribution (CC-BY) license (http://creativecommons.org/licenses/by/4.0/).