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# Proceeding Paper Simulation of 1-d Solute Transport with Equilibrium-controlled Non-linear Sorption using MT3DMS <sup>+</sup>

Amit Kumar 1\*, Aftab Alam 1 and Anshuman Singh 2.

1*	Department of Civil Engineering, National Institute of Technology Patna, Bihar 800005, India;
	amitkr.ph21.ce@nitp.ac.in.

- <sup>1</sup> Department of Civil Engineering, National Institute of Technology Patna, Bihar 800005, India; aftaba.phd19.ce@nitp.ac.in
- <sup>2</sup> Department of Civil Engineering, National Institute of Technology Patna, Bihar 800005, India; asingh@nitp.ac.in
- \* Correspondence: amitkr.ph21.ce@nitp.ac.in; Tel.: (+91 8871007952)
- + Presented at the title, place, and date.

Abstract: Groundwater contamination is a rising worldwide issue, and it must be treated well as 13 most of the world relies on it. Groundwater pollution occurs when undesirable substances in 14 groundwater rise. Understanding, simulating, and predicting solute mobility in groundwater helps 15 treat polluted groundwater. MT3DMS has been used to model contaminant movement with a non-16 linear Freundlich sorption isotherm. MT3DMS stands for "Modular three-dimensional multispecies 17 transport model". MT3DMS software has several categories of solute transport solution techniques 18 like FDM and the higher-order finite-volume TVD method in a unique single code. Applying the 19 combination of these solution techniques is believed to give the best possible solution with greater 20 precision and accuracy. In the current work, the benchmark problem (P2) of the MT3DMS package 21 was taken, and the chemical reaction package was modified according to our problem. Multiple 22 simulations were run with different adsorption capacities and intensities, incorporating the nonlin-23 ear Freundlich sorption isotherm. After that analysis of BTC trends, at a position 8 cm from the 24 source, the pulse input of contamination was discharged for 160 seconds. The simulation lasted 1500 25 seconds. The observation output files were imported to plot BTCs for trend analysis and visualize 26 simulation results. After comparing the various BTCs, it was found that the adsorption capability 27 of porous medium enhances retention capacity, so contaminants are sorbed and retarded by the 28 solid phase more, slowing the contaminant movement and delaying the BTC peak. For similar ad-29 sorption capacity at lower adsorption intensity, the solid retains more contaminant and the peak is 30 attenuated as well as delayed, but as the adsorption intensity increases, the relative concentration 31 in the aqueous phase increases, and the peak is enhanced early as the solid retains less contaminant. 32

Keywords: Groundwater contamination; Sorption; MT3DMS; Simulation; Breakthrough curves 33 (BTC) 34

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1. Introduction

Groundwater pollution is one of the major emerging challenges globally [1]. Since 37 groundwater is a potential source of water for use in daily life and for basic usages like 38 drinking, it should be strongly considered for assessment and the design of remedial tech-39 niques for its pollution [2]. Generally, it is assumed that the migration of a reactive solute 40 in any porous media with groundwater is assumed to be driven by a linear sorption iso-41 therm [3]. Though research efforts are focused on determining the non-linear isotherms' 42 functional structure, principles, and features, often hydrologists and environmental engi-43 neers are also engrossed in predicting the plume propagation and breakthrough curves 44 for multiple or a single set of contamination schemes and scenarios. For the present study, 45

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**Copyright:** © 2023 by the authors. Submitted for possible open access publication under the terms and conditions of the Creative Commons Attribution (CC BY) license (https://creativecommons.org/licenses/by/4.0/). we will take the pulse concentration source with a single contaminant for a better under-1 standing of its fate and transport with nonlinear sorption. The processes responsible for 2 the transport of solutes in groundwater as per [4] are advection, mechanical Dispersion, 3 molecular diffusion, sorption and colloid transport. [5] They used HYDRUS 1D/2D soft-4 ware for the simulation of solute transport, making variations in critical parameters of 5 flow and transport for various soil conditions. [6] The study was conducted for the reme-6 diation of metal contaminants in groundwater and greywater. Heavy metals were ex-7 tracted efficiently from test samples using the adsorption process. 8

## 2. Methodology

#### 2.1. MT3DMS

For this study, the simulation was done with the help of MT3DMS software down-11 loaded from the website of the U.S. Geological survey department 12 (<u>https://www.usgs.gov</u>). MT3DMS can be used to simulate various physical and chemical 13 changes undergoing solute transport processes. Also, there is independence in the selec-14 tion of different boundary conditions. The basic chemical reaction package named RCT 15 can solve single species with conditions like equilibrium-controlled or non-linear sorption 16 and 1st order kinetic reactions. 17

Governing equation of solute transport model in MT3DMS is:

$$R\theta \frac{\partial \bar{c}}{\partial c} = \frac{\partial}{\partial x_i} \left\{ \theta D i j \frac{\partial c}{\partial x_j} \right\} - \frac{\partial}{\partial x_i} (\theta v_i C) + q_s C s - q'_s C - \lambda_1 \theta C - \lambda_2 \rho_b \bar{C}$$
(1) 19

 $(\theta = \text{Porosity of media, dimensionless; C} = \text{Dissolved concentration in aqueous}$ 20 phase;  $ML^{-3}$ , t = time; Seconds,  $V_i$  = seepage velocity;  $LT^{-1}$ ,  $q_s$  = discharge per unit volume 21 of medium; T<sup>-1</sup>, q's = rate of change in GW storage; T<sup>-1</sup>,  $\rho_b$ =bulk density medium; ML<sup>-1</sup>,  $\bar{C}$ = 22 sorbed contaminant concentration on the medium;  $MM^{-1}$ ,  $\lambda_1 = 1^{st}$  order reaction rate for 23 dissolved phase; T<sup>-1</sup>,  $\lambda_2$  =1<sup>st</sup> order reaction rate for the sorbed phase, T<sup>-1</sup>; R = Retardation 24 factor) 25

#### 2.2. Chemical reaction package

For the simulation in MT3DMS, the assumption of equilibrium condition is made 27 generally, that sorption is instantaneous and the sorption reaction is faster as compared 28 to seepage velocity. 29

As we know, at constant temperature, the sorption process is explained by the func-30 tional relationship between sorbed and dissolved concentrations. It is termed the sorption 31 isotherm. The retardation factor (R), described in governing equation (1), is commonly 32 used to integrate equilibrium-controlled sorption isotherms into the transport model. 33 Since, this study incorporates the non-linear Freundlich sorption isotherm, The empirical 34 equation for the same is: 35

$$\bar{c} = K_f c^a \tag{2} \qquad 36$$

(K<sub>f</sub> = Freundlich constant (Adsorption capacity/partition factor), ( $L^{3}M^{-1}$ ); a = Freun-37 dlich exponent (adsorption intensity) dimensionless.).

Retardation factor can be defined as:

$$R = 1 + \frac{\rho_b}{\theta} \frac{\partial \bar{c}}{\partial c} = 1 + \frac{\rho_b}{\theta} a K_f C^{a-1}$$
(3) 40

#### 2.3. Transport model Formation and Validation

For this study, the same problem is opted as demonstrated in [7], considering Freun-42 dlich isotherms, and the input files for the same problem are given in benchmark problem 43 P2 of the MT3DMS software. For the creation of input files according to our study, desired 44 changes in existing input files of the chemical reaction package of MT3DMS input files 45 (RCT package) were made, and modifications were made as per directions suggested in 46 the MT3DMS user manual [8] for the creation of desired input files. 47

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Model parameters were taken as follows: Grid-space( $\Delta X$ ) = 0.160 cm, Dispersivity( $\alpha_L$ ) = 1.0 cm, GW seepage-velocity(v)= 0.1 cm/s, Porosity( $\theta$ ) = 0.37, Bulk density( $\varphi$ b) =1.587 g/cm3, Source concentration (Co) = 0.05 mg/l, Pulse Input time (t<sub>o</sub>)= 160 seconds. Here, due to the low Peclet number (0.16), the transport mechanism will not be ad-

vection-dominating and the migration of the solute will be due to sorption.

Initial and boundary conditions for our model are provided as follows:

C(x, 0) = 0(4)7 (fo  $0 < t < t_0$ (ac)

$$-\theta D\left(\frac{\partial c}{\partial x}\right) + qC\left[(x=0) =\begin{cases} f & 0 < t < t0\\ 0 & t > to \end{cases}$$
(5) 8

$$\frac{\partial C}{\partial x}(\infty, t) = 0 \qquad \qquad t > 0 \qquad \qquad (6) \quad 9$$

The boundary conditions for the flow model in this study involve setting constant 11 hydraulic head values at both ends of the model domain. This is done by assigning an 12 arbitrary hydraulic head value to achieve the desired Darcy flux. As the flow field is 13 steady-state, only one stress period is necessary for the flow model. However, the 14 transport model requires two stress periods to account for changes in source concentra-15 tion. 16

In the transport model used in this study, the boundary conditions involve a specified total mass flux (third-type) on the left side and zero dispersive mass flux (secondtype) on the right side. To approximate the third-type boundary condition, the advective mass flux is specified using the rate of inflow or outflow across the boundary (represented by q). To implement this in the test problem, the concentration of the inflow at the constant-head node on the left is set to 0.05 mg/L for the first and second stress periods, respectively. The second-type boundary condition is addressed by placing it far enough from the source to prevent the plume from reaching it within the given simulation time.

The validation of the model was done with the benchmark problem, and the result obtained from the analytical method were like the numerical simulation.

### 2.4. Input files, Data and Simulation

Multiple simulations were run for this problem by altering the values within the 28 range discussed below, and the output of each simulation was saved individually to avoid 29 any possible error. The Chemical reaction input files package (RCT package) incorporat-30 ing the nonlinear Freundlich sorption isotherm has two sorption parameters, SP<sub>1</sub>=K<sub>f</sub> and 31 SP<sub>2</sub>= a, and the input data variation was made with different sets of these two parameters 32 as follows: 33

Set number-1	$K_{\rm f}$ =0.20 to 1.40 & a=0.70,	34
Set number-2	$K_f = 0.20$ to 1.40 & $a=0.60$ ,	35
Set number-3	$K_f = 0.20$ to 1.40 & $a=0.80$ .	36
For the variatic	on for 'K <sub>f</sub> ' (SP <sub>1</sub> ) and 'a' (SP <sub>2</sub> ) for above sets, the range for variation has	37

For the variation for 'K<sub>f</sub>' (SP<sub>1</sub>) and 'a' (SP<sub>2</sub>) for above sets, the range for variation has been taken from [9], [10], [11] and [12]. All other input parameters and boundary condi-38 tions of the model remain unchanged and are applied as per the standard benchmark 39 problem. 40

#### 3. Result and Discussion

All the outputs were obtained using a concentration pulse input for 160 seconds with 42 an initial concentration (Co) of 0.05 mg/l and a total simulation length of 1500 seconds. 43 The BTCs are displayed at a location 8 cm from the initial source point of the pulse input. 44 By changing the values of the parameters ( $K_f$  and a), a trend analysis for the BTCs (C/Co 45 vs simulation time) for three alternative sets of parameters is generated with the help of 46 MS Excel. Here; C/Co = aqueous phase relative concentration. 47

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Figure 1: - when, a= 0.7, comparison of breakthrough curves for different Kévalues.

It is being observed that with the same adsorption intensity (a), as the value of adsorption capacity increases, there is an attenuation in the peak of relative phase aqueous concentration since retardation to contaminants is offered by the solid phase (figure 2).





With a lower value of a= 0.6 and varying K<sub>t</sub>, it is observed that the relative phase 10 aqueous concentration drastically reduces and the peak is also delayed for each set since 11 more resistance is offered to the contaminant (figure 4). 12

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Figure 5: - when, a= 0.8, comparison of breakthrough curves for different Kt values.

With a higher value of a= 0.8 and by varying K<sub>t</sub> it is observed that the relative phase aqueous concentration is increased and the peak occurs earlier as compared to previous results (figure 6).

Also, it has been absorbed that at the same 'a' value and with an increase in  $K_f$  the peak is delayed and attenuated (aqueous phase relative concentration decreases with an increase in  $K_f$ ).

#### 4. Conclusion

A porous medium's ability to adsorb contaminants from an aqueous solution is di-10 rectly proportional to its adsorption capacity. As a result, the concentration of contami-11 nants in the aqueous phase decreases when the adsorption capacity of the porous medium 12 is higher, since more of the contaminant is adsorbed onto the solid phase. This leads to a 13 reduction in the peak relative aqueous phase concentration and a slower rate of pollutant 14 migration. However, a higher adsorption intensity can have an adverse effect on pollution 15 migration. When the adsorption intensity is high, the concentration of contaminants in the 16 aqueous phase increases compared to the concentration at a lower adsorption intensity, 17 even when the adsorption capacity is the same. These critical parameters can be used to 18 design a customized media with a suitable pollution removal process to treat targeted 19 pollution. 20

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