

Proceeding

# Experimental investigation on crack growth in pre-notched concrete beams<sup>†</sup>

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**Abstract:** It is well-known that failure of unreinforced, pre-notched concrete beams in bending is mainly governed by the tensile strength (“ductile” behaviour) or by the fracture toughness (brittle behaviour) depending on the geometrical scale (beam size), and the value of the relative notch depth. In particular, relatively large beams made of high-strength concrete and with a small relative notch depth, show a brittle structural behaviour (unstable crack propagation); whereas relatively small beams made of low-strength concrete and with a large relative notch depth, show a relatively ductile structural behaviour (stable crack propagation). In this contribution, the damage progress, due to crack formation and propagation, in unreinforced, pre-notched concrete beam specimens, tested in three-point bending, is analysed by the Acoustic Emission (AE) and Digital Image Correlation (DIC) techniques. Beams with rectangular cross-section were considered. A relative notch depth ( $a/d$ ) equal to 0.5 is assumed in all cases. The loading process was operated by controlling the vertical displacement. The specimens were instrumented by four transducers measuring the vertical displacements in correspondence to the supports and at mid-span, and the Crack Mouth Opening Displacement (CMOD). In addition, two AE transducers were located near the notch to acquire the AE signals originated by material damage. Moreover, the evolution of the deformation process was monitored around the notch by a DIC system, therefore obtaining the time evolution of strains. By analysing the acquired data, a correlation between the AE signals registered and the displacements/strains measured at several points was looked for. Furthermore, the fracture energy of each specimen was evaluated, according to RILEM recommendation, based on the measured load–deflection curves. Scale effects on fracture energy, bending strength, and AE energy per unit area were investigated.

**Keywords:** pre-notched concrete beams, crack propagation, acoustic emission, digital image correlation.

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## 1. Introduction

In the present work, three-point bending tests were performed on plain concrete pre-notched specimens of different sizes, investigating the damage process that takes place when the dimensional scale is increased, maintaining all the remaining parameters unchanged [1]. RILEM Recommendation [2,3] was followed for the testing procedure and for the evaluation of Fracture Energy and bending strength. During the tests, two non-destructive techniques for damage monitoring were used: Acoustic Emission (AE) analysis and 2D Digital Image Correlation (DIC).

As is well-known, the AE technique is based on the acquisition of the ultrasonic signals generated by crack formation and extension in the bulk of the material [4-6]. Unlike Fracture Energy, that is dissipated in an invasive fractal domain, AE energy seems to be emitted in a lacunar fractal

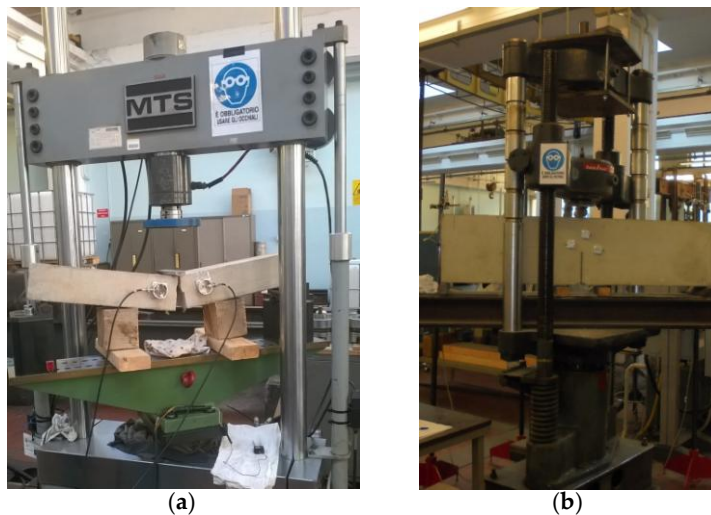
domain: this difference in the fractal physical dimensions indicates the un-correlation between dissipated and emitted energies [7-9].

On the other hand, DIC is an innovative non-contact optical technique for measuring displacement and strain based on accurate 2D and 3D tracking and registration of changes in images [10,11].

For three-point bending testing, specimen sizes prescribed by [2,3] were taken into account (Table 1). Specimens were tested by servo-hydraulic universal testing machines: an MTS with a maximum loading capacity of 100 kN was used for testing the smaller beams (depth = 100 mm); the tests on the beams of larger sizes were conducted – for convenience in terms of experimental set-up – using a Baldwin testing machine with maximum loading capacity of 500 kN (Figure 1). Three tests were conducted on specimens with  $d = 100$  mm; two tests on beams with  $d = 200$  mm; one test on a beam with  $d = 300$  mm; and three tests on beams with  $d = 400$  mm. The mechanical properties of the concrete constituting the beams are the following: mass density  $\rho = 2,310$  kg/m<sup>3</sup>; cubic compression strength  $R_c = 26.4$  MPa; cylindrical compression strength  $f_c = 21.9$  MPa; average tensile strength  $f_{ct} = 2.4$  MPa; average elastic modulus  $E_c = 30,570$  MPa.

**Table 1.** Specimen sizes considered in the experimental campaign according to [2,3].

$D_{max}$ of aggregate (mm)	Depth $d$ (mm)	Width $b$ (mm)	Length $L$ (mm)	Span $S$ (mm)
1 – 16	$100 \pm 5$	$100 \pm 5$	$840 \pm 10$	$800 \pm 5$
16 – 32	$200 \pm 5$	$100 \pm 5$	$1190 \pm 10$	$1130 \pm 5$
32 – 48	$300 \pm 5$	$150 \pm 5$	$1450 \pm 10$	$1385 \pm 5$
48 – 64	$400 \pm 5$	$200 \pm 5$	$1640 \pm 10$	$1600 \pm 5$



**Figure 1.** Experimental set-up for three-point bending tests on universal testing machines: (a) specimen with  $d = 100$  mm on MTS machine after final failure; (b) testing set-up on Baldwin machine for specimens with  $d = 200, 300,$  and  $400$  mm.

All the specimens were tested until the final failure was reached. During the tests, Crack Mouth Opening Displacement (CMOD) was measured using a clip-on strain gage applied to the notch mouth; the vertical displacement of the beam mid-section was taken equal to the stroke of the machine hydraulic jack. Piezoelectric resonant AE sensors were applied on the specimen surface in order to capture AE signals [5]. The data were acquired by an 8-channel national instruments

digitizer, setting the acquisition threshold to 2 mV, and adopting a sampling frequency of 1Msample/s. The registered signals were amplified by 60 dB before processing.

On the other hand, DIC measurements were performed on two beams, one with  $d = 300$  mm and the other with  $d = 400$  mm, by means of VIC-2D system by Correlated Solutions. Figure 2a shows the set-up of a test with the DIC apparatus, whereas Figure 2b shows a detail around the mid-span section, cracked after final collapse.

In the following, we briefly report on some of the results obtained. More details will be given in future contributions.

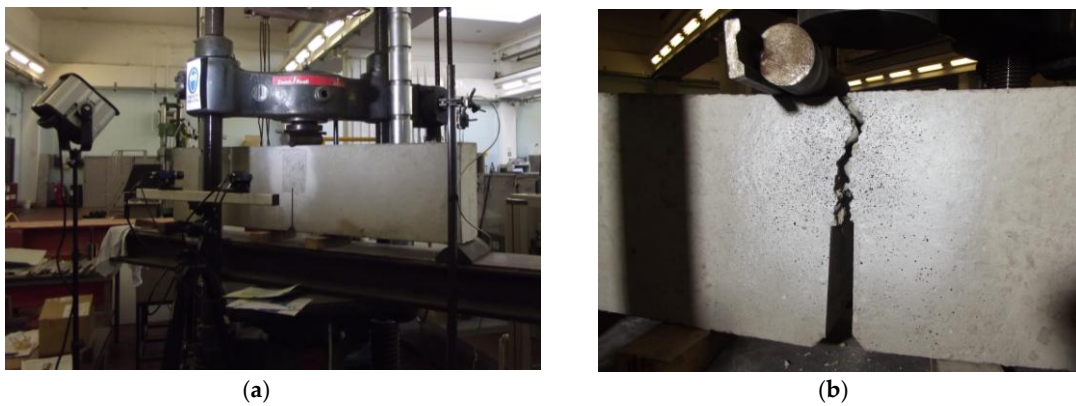
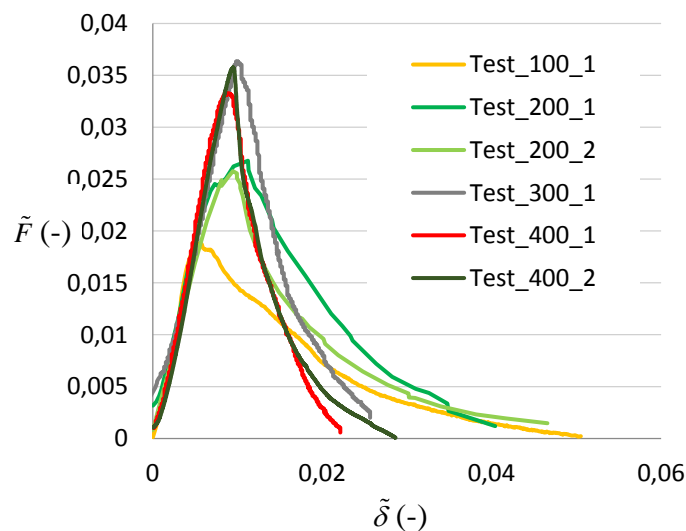


Figure 2. (a) Experimental set-up for DIC measurements; (b) mid-span region after collapse.

## 2 Results

In order to define scale-invariant material properties, the fractal nature of the damaged microstructure in quasi-brittle and disordered materials must be considered [12-14]. It is known that: for tensile (bending) strength, the actual dimension of material ligament at peak stress is  $\Delta_\sigma < 2$  (lacunar domain); for fracture energy, the actual dimension of fracture surface at failure is  $\Delta_G > 2$  (invasive domain). An interesting result is obtained here for AE energy per unit area, i.e., it is dissipated on a lacunar fractal domain ( $\Delta_{AE} < 2$ ).

Figure 3 shows the renormalized load–deflection curves. The load  $F$  is nondimensionalised as:  $\tilde{F} = F/(\sigma_u^* b d^{1-d_\sigma})$ , where  $\sigma_u^*$  is the renormalized tensile (bending) strength and  $d_\sigma$  is the corresponding fractal exponent. The deflection  $\delta$  is nondimensionalised as:  $\tilde{\delta} = \delta/d^{1-d_\sigma}$ . The value of  $d_\sigma$  is, in this case, equal to 0.36, as will be shown hereafter.

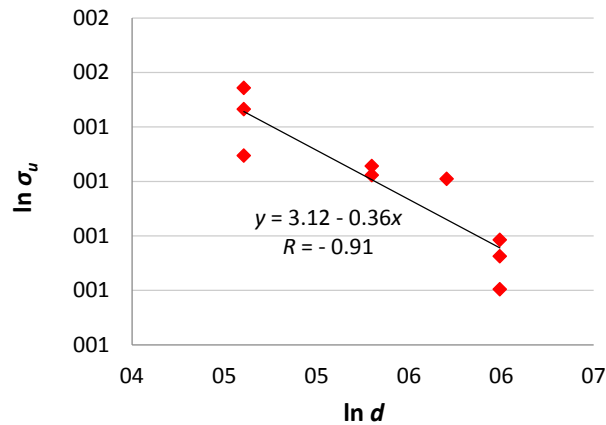


**Figure 3.** Renormalized load vs. deflection curves.

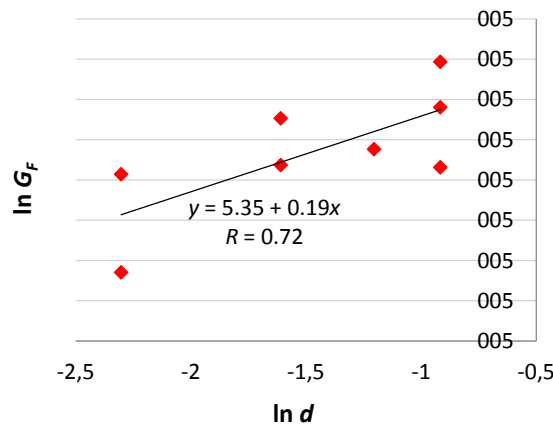
In Figure 4, the bending strength vs. specimen size (beam depth) values, corresponding to all the tests, are reported in a bi-logarithmic diagram. The relevant fractal scaling law reads:  $\ln \sigma_u = \ln \sigma_u^* - d_\sigma \ln d$ , with  $d_\sigma = 0.36$ ,  $\sigma_u^* = 22.6 \text{ Nmm}^{-1.64}$  ( $\Delta_\sigma = 2 - d_\sigma = 1.64$ ).

Figure 5 shows the fracture energy vs. beam depth diagram. The corresponding fractal scaling law is  $\ln G_F = \ln G_F^* + d_G \ln d$ , with  $d_G = 0.19$ ,  $G_F^* = 209.9 \text{ Jm}^{-2.19}$  ( $\Delta_G = 2 + d_G = 2.19$ ).

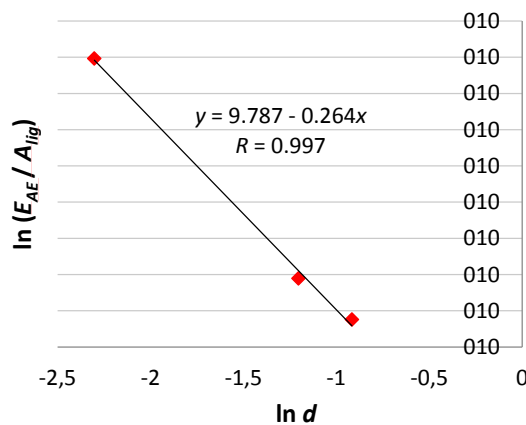
Figure 6 shows the AE energy per unit area vs. beam depth diagram. The fractal scaling law is  $\ln \bar{E}_{AE} = \ln \bar{E}_{AE}^* - d_{AE} \ln d$ , with  $d_{AE} = 0.26$ ,  $E_{AE}^* = 17,800 \text{ msVm}^{-1.74}$  ( $\Delta_{AE} = 2 - d_{AE} = 1.74$ ).



**Figure 4.** Bending strength vs. beam depth diagram ( $R$  is the Pearson correlation coefficient).

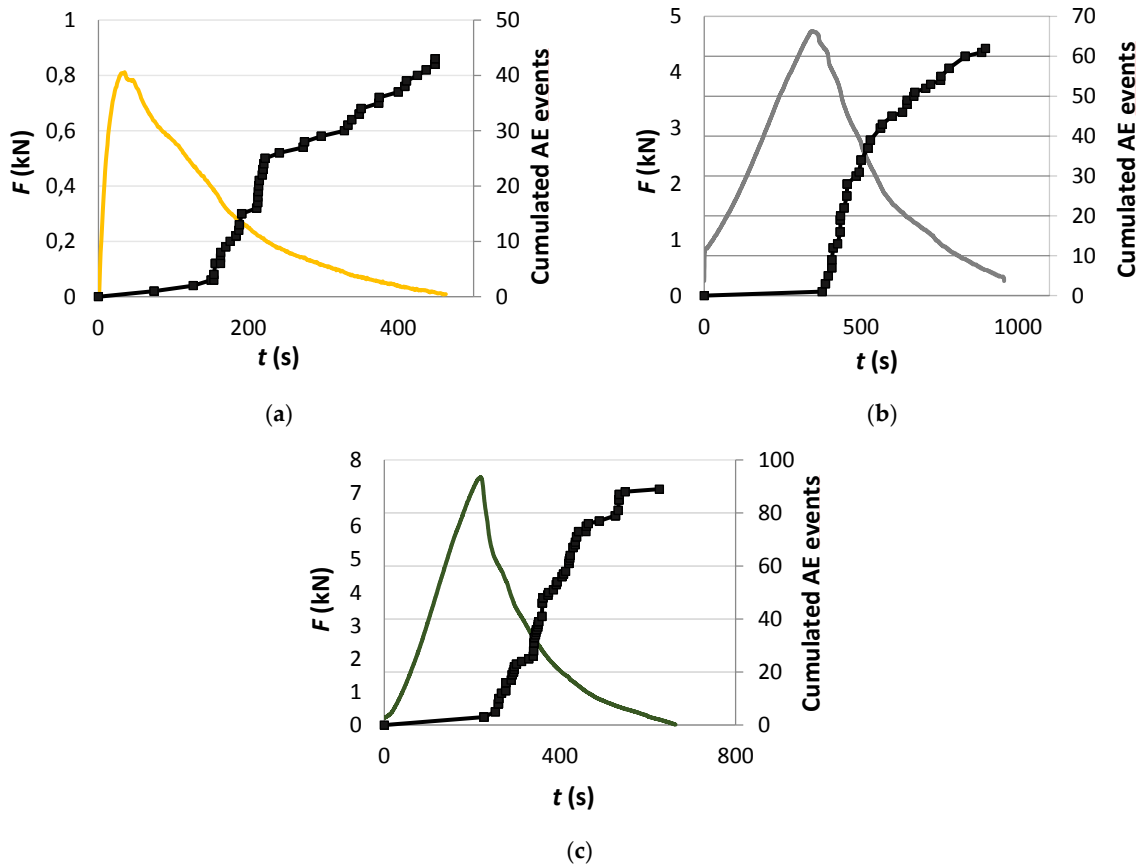


**Figure 5.** Fracture energy vs. beam depth diagram ( $R$  is the Pearson correlation coefficient).



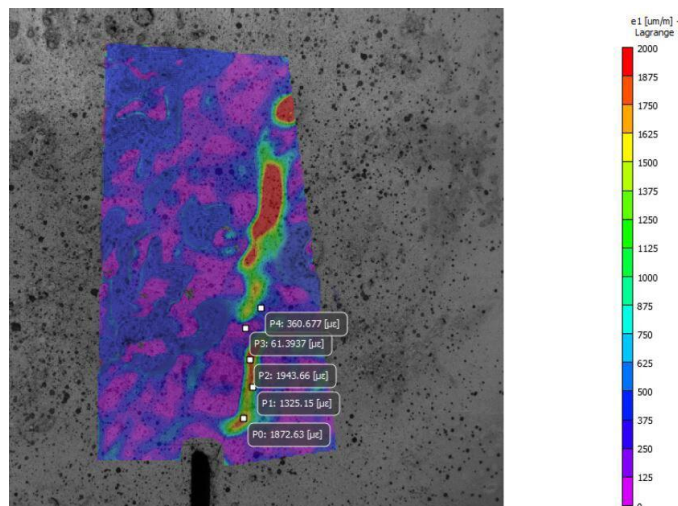
**Figure 6.** AE energy per unit area vs. beam depth diagram ( $R$  is the Pearson correlation coefficient).

In Figure 7, the load and cumulated AE energy vs. time diagrams of three tests are superimposed.



**Figure 7.** Load and cumulated AE events vs. time for (a)  $d = 100$  mm, (b)  $d = 300$  mm, (c)  $d = 400$  mm.

Lastly, the time evolution of strains at several points around the pre-notched section, and in particular along the crack, was obtained from DIC results. An example of data is shown in Figure 8, where principal strain values at five points (P0-P4) are superimposed to the relevant strain contour plot (along principal direction 1), for a test made on a beam with  $d = 400$  mm. In future works, the damage progress monitored by DIC will be correlated with RA (AE signal rise time/the peak amplitude), AF (AE ringdown counts/ the duration time),  $b$ -value, and other AE parameters.



**Figure 8.** Example of contour plot and strain values along principal direction 1 obtained by DIC.

## 4 Conclusions

Scale effects on fracture energy, bending strength and AE energy per unit area were observed by performing three-point bending tests on plain concrete specimens of four different sizes. The fractal model was applied to define scaling laws and renormalized scale-invariant values for the above-mentioned parameters. Results indicate the absence of correlation between dissipated (fracture) and emitted (AE) energies. The damage process around the pre-notched section was monitored by AE and DIC techniques, thus offering the possibility to correlate the two sets of data. Deeper investigations on the latter point will be the objective of future contributions.

**Author Contributions:** LACIDOGNA, PIANA, ACCORNERO, CARPINTERI performed the experiments, analyzed the data, and wrote the paper.

**Conflicts of Interest:** The authors declare no conflict of interest.

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