

1 Congress Communication

2 Energy dissipation structures: Influence of aeration 3 in supercritical flows

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9 **Abstract:** Adequate design of energy dissipation structures is essential for effective flood control.
10 The effect of aeration on water flow has been one of most analyzed phenomena during the last
11 decades due to its influence on hydraulic structures. The purpose of this study is to characterize the
12 influence of aeration on the boundary friction in supercritical and fully turbulent flows. Our analysis
13 is based on a physical model to reproduce these phenomena and consists of a spillway chute 6.5 m
14 high followed by a 10 m length and 2 m high still basin. Water and air are supplied by a pump and
15 compressors and controlled at the entrance by several valves and flowmeters and the channel is
16 monitored to measure the velocity profile and air concentration in the intake flow to the still basin.
17 Velocity results included in this paper show the relation between air concentration and energy
18 dissipation by friction. To determine this relation, Manning roughness numbers have been obtained
19 for all scenarios. It has been found that greater air entrainment implies acceleration of the flow, since
20 friction is the main energy dissipation mechanism in open channels flow.

21 **Keywords:** aeration; velocity; concentration; energy dissipation; friction; Manning.

22 **PACS:** J0101

23

24 1. Introduction

25 The social and economic impact of floods represents a very important issue due to the enormous
26 amounts of losses involved. In this context, dams play a crucial role to match the requirements for
27 hydrological regulation against flooding phenomena, especially in basins with extreme hydrological
28 regimes. This is the reason why hydraulic and dam operation requirements need to cope with
29 increasing safety standards and the future dam technical regulation should include the need for
30 higher levels of dam operation control and safety devices, as well as higher discharge capacities. In
31 this context, the need to review the hydraulic capacity of current dams (including weirs, spillways
32 and sluices) is clear, considering all the effects of the flow over the dam structure. In this sense, one
33 structural element that strongly affects the discharge capacities in weir gravity dams is the stilling
34 basin, which is clearly a hydraulic device for energy dissipation structure with a high cost, limited
35 design boundaries and operation conditions. Currently, the stilling basin design depends on
36 hydraulic variables of intake flow. Our research includes the influence of aeration in the energy-
37 dissipation ratio in this analysis. The Hydraulic Laboratory of CEDEX (Spain) is carrying out an
38 experimental study of the aeration influence over chutes and stilling basins in the framework of the
39 EMULSIONA project, a research funded by the Spanish Ministry of Economy. Our analysis is based
40 on a 1:1 scale physical model designed to reproduce different scenarios with water and air flow rates.
41 The experimental works are organized into two stages. First, the analysis is focused on the effects of
42 aeration over the supercritical flow in the channel and how the velocity field is modified according
43 to different air concentrations. The second part is currently being carried out and is aimed at
44 characterizing the evolution of the hydraulic jump determined by the spillway channel conditions.

45 The aim of this paper, reporting the first stage of the research, is to analyse the effects of the aeration
 46 over the flow and to characterize its influence over the boundary friction.

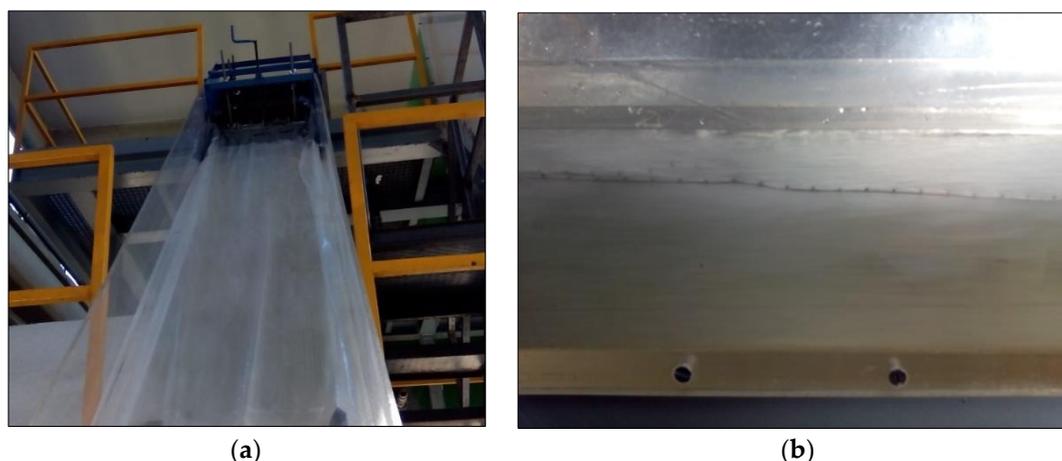
47 2. Results

48 Results obtained during the experimental phase involve a total of 12 scenarios of air (Q_a) and
 49 water (Q_w) flow (Table 1). Air flow is supplied by a compressor and is controlled by a valve and a
 50 flowmeter before the mixing air-water device. Table 1 shows also the average velocity (V_{in}) and air
 51 concentration (C_{in}) in the physical model test entrance, with section width of 0.5 m and height of 0.08
 52 m.

53 **Table 1.** Experimental scenarios with average velocity and air concentration at the intake channel.

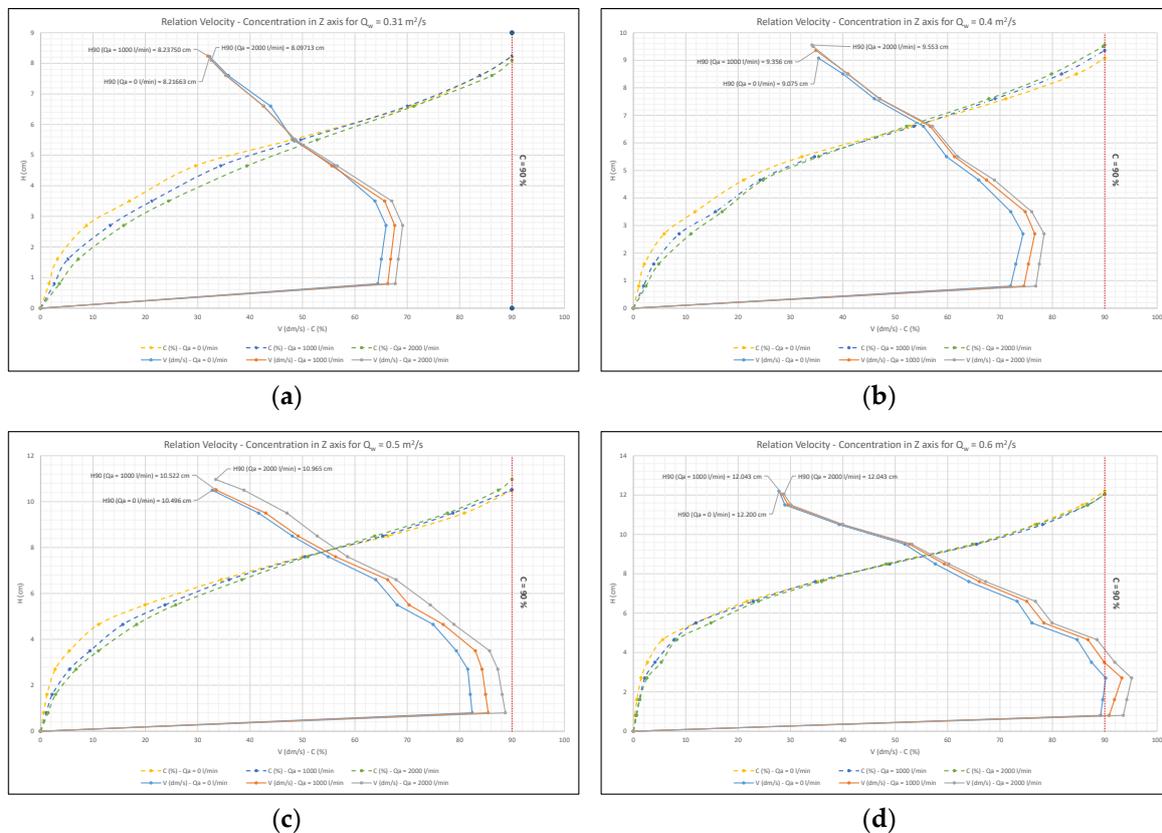
Scenario	Q_w (m ² /s)	Q_a (l/minute)	V_{in} (m/s)	C_{in} (%)
1.1		0	3.8750	0
1.2	0.31 (155 l/s)	1000	4.3045	9.9778
1.3		2000	4.7391	18.2338
2.1		0	5	0
2.2	0.4 (200 l/s)	1000	5.4500	8.2569
2.3		2000	5.9120	15.4258
3.1		0	6.2500	0
3.2	0.5 (250 l/s)	1000	6.7182	6.9692
3.3		2000	7.2063	13.2708
4.1		0	7.5000	0
4.2	0.6 (300 l/s)	1000	8.0046	6.3034
4.3		2000	8.5288	12.0631

54 The main goal of the experimental work is to measure velocity and air concentration profiles in
 55 the final section of the spillway channel, just upstream of the stilling basin. These results are necessary
 56 to characterize the hydraulic jump inflow and the energy dissipation in the channel by boundary
 57 friction, evaluating at the same time the aeration influence on the energy dissipation processes. To
 58 reproduce a real condition of fully turbulent flow, the channel is covered at the top by a metallic mesh
 59 to increase the turbulence along the channel. This element is flexible and does not hinder the free
 60 flow. On the other hand, a flexible plastic cover has is set over the channel to reduce the air exchange
 61 between flow and atmosphere (Figure 1).



62 **Figure 1.** (a) Border conditions over the flow surface during the experimental analysis; (b) Effects of
 63 the metallic mesh and plastic cover over the flow in test

64 Figure 2 and Table 2 show the relation between velocity (V_{Out}) and air concentration profiles
 65 (C_{Out}) in the channel exit section. Moreover, results includes the depth of the experiments when
 66 concentration of 90% is reached ($H_{90 Out}$), a value usually considered in the related scientific literature.



67 **Figure 2.** Relation between velocity and concentration profiles: (a) Scenario 1 ($Q_w = 0.31 \text{ m}^2/\text{s}$); (b)
 68 Scenario 2 ($Q_w = 0.4 \text{ m}^2/\text{s}$); (c) Scenario 3 ($Q_w = 0.5 \text{ m}^2/\text{s}$); (d) Scenario 4 ($Q_w = 0.6 \text{ m}^2/\text{s}$).

69 **Table 2.** Average velocity, concentration and H_{90} value at the channel exit.

Scenario	V_{Out} (m/s)	C_{Out} (%)	$H_{90 Out}$ (cm)
1.1	5.1874	29.2792	8.2166
1.2	5.2404	31.2996	8.2375
1.3	5.3541	33.0260	8.0971
2.1	5.8814	27.0491	9.0750
2.2	5.9790	29.5446	9.3555
2.3	6.0255	30.3674	9.5525
3.1	6.3162	23.9025	10.4956
3.2	6.5179	25.7145	10.5220
3.3	6.6851	27.8937	10.9652
4.1	6.5939	22.1556	12.2004
4.2	6.8136	22.4595	12.0333
4.3	6.9479	22.8837	12.0434

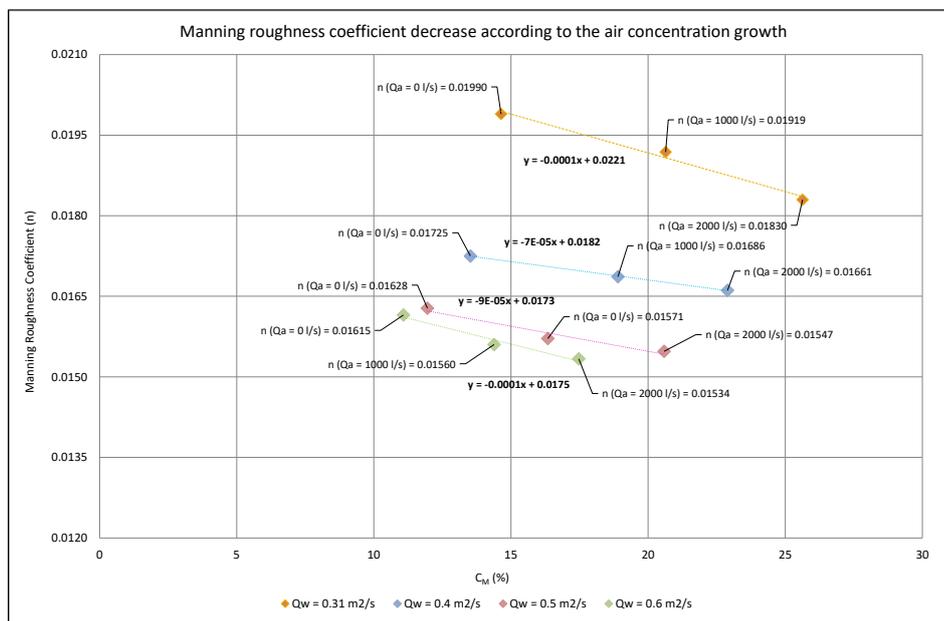
70 **3. Discussion**

71 The results obtained during the experimental phase show that, in a constant water flow, there is
 72 a velocity increase with the aeration growth. There are different energy dissipation mechanism in
 73 spillways, but the most important in open channel flow is the contour friction. Considering all the
 74 methods to evaluate this effect, Manning [1] formulation has been chosen because it is well known

75 and widely used in the hydraulic engineering area to determine the friction slope (I_f) based on a
 76 roughness coefficient n (1), where V represents the average velocity and R_h the hydraulic radius.

$$I_f = \frac{n^2 V^2}{R_h^{2/3}} \quad (1)$$

77 The aeration affects the energy dissipation mechanisms in different ways. Hinze [2]
 78 considers that aeration increases the viscous turbulent dissipation but this formulation is theoretical
 79 and without empirical support. Other authors consider the division and reunification of bubbles as
 80 the main factor over energy losses [3-5]. In this case, shear stress between flow layers breaks the
 81 bubbles to regroup each other's in collision areas later. This process has to exceed the surface tension
 82 of the air particles and generates energy dissipation by heat. Both methods are opposed to the
 83 Manning formulation because they consider the turbulence as main effect of dissipation instead of
 84 roughness. In our experimental case, we used the first option (Manning) to analyze the energy
 85 dissipation due to contour friction, which is prevailing in supercritical flows with low water depth
 86 and high velocity. The application of other formulations would be interesting during the analysis of
 87 the hydraulic jump, where turbulence effects are more important over the flow. Using the velocity
 88 and air concentration profiles in the initial and final sections of the channel, it is possible to calculate
 89 the average values that characterize the spillway flow (V_M, C_M, H_{90M}) and also the friction slope of our
 90 test stretch. Including these data in Manning equation (1), a representative Manning roughness
 91 coefficient (n) is obtained for each scenario (Table 3). Moreover, this table includes as well the
 92 reduction rate in % (Δn) of Manning coefficient with respect to the roughness without aeration. Figure
 93 3 relates the Manning roughness coefficient (n) with each concentration (C_M) and demonstrates a
 94 roughness reduction with an air concentration increase.



95

96 **Figure 3:** Relation between Manning roughness coefficient (n) and average air concentration (C_M) for
 97 all scenarios.

98

Table 3. Average velocity, concentration, H_{90} and n value at the middle section of channel.

Scenario	V_M (m/s)	C_M (%)	H_{90M} (cm)	n	Δn (%)
1.1	4.5312	14.6396	8.1083	0.01990	9.8119
1.2	4.7725	20.6387	8.1187	0.01919	13.0232
1.3	5.0466	25.6299	8.0485	0.01830	17.0461

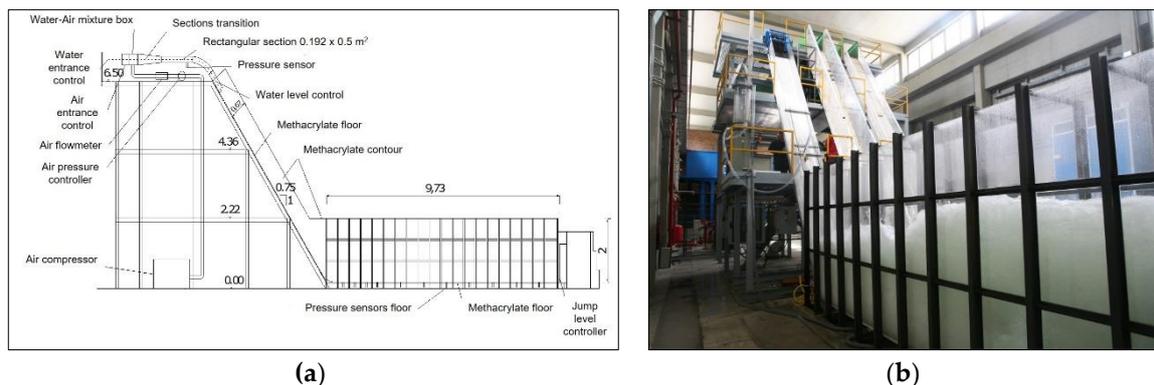
Scenario	V_M (m/s)	C_M (%)	$H_{90 M}$ (cm)	n	Δn (%)
2.1	5.4407	13.5246	8.5375	0.01725	5.0600
2.2	5.7145	18.9007	8.6777	0.01686	7.1881
2.3	5.9687	22.8966	8.7762	0.01661	8.5816
3.1	6.2831	11.9512	9.2478	0.01628	5.5774
3.2	6.6181	16.3419	9.2610	0.01571	8.8573
3.3	6.9457	20.5823	9.4826	0.01547	10.2429
4.1	7.0469	11.0778	10.1002	0.01615	7.8570
4.2	7.4091	14.3815	10.0216	0.01560	10.9981
4.3	7.7384	17.4734	10.0217	0.01534	12.5198

99 4. Materials and Methods

100 4.1. Physical model

101 The experimental device consists of a spillway chute 6.5 m high, 0.5 m wide and slope of 75%,
 102 followed by a 10 m length and 2 m high stilling basin where the hydraulic jump is confined. Water
 103 and air are supplied by a pump and compressors and controlled at the entrance by several valves
 104 and flowmeters.

105 The maximum flow rates are 0.6 m²/s of water and 2000 l/min of air. Under these conditions,
 106 tested velocity ranges vary between 5 and 7 m/s with Froude number between 5 and 6.5. Figure 4
 107 shows a general scheme of the installation and a general view picture of the physical model.



108 **Figure 4.** Scheme of the physical model: (a) Section of the experimental structure with supply
 109 machines and contour materials; (b) Frontal view of the spillway channel and stilling basin.

110 4.2. Data collections methods

111 The two flow variables measured during the tests were the velocity and concentration profiles
 112 in exit section of the channel. The flow velocity was collected by means of a Pitot probe with a
 113 pressure sensor and connected to a data acquisition program developed in CEDEX with LabVIEW.
 114 The acquisition frequency is 100 data/s and the recording time reaches 100 s.

115 The second method was focused on collecting the concentration data. In this case, the instrument
 116 used was an Air Concentration Meter (ACM) developed by the Hydraulic Engineering Department
 117 of the Universidad Politécnica de Cartagena (UPCT). This probe is based on a prototype developed
 118 in 1997 by U.S. Department of the Interior Bureau of Reclamation [6] and is designed to measure the
 119 percentage of air entrained in flowing water. This methodology detects the air bubbles passing
 120 through the water by changes in conductivity that take place when a bubble impinges on the probe
 121 tip. Figure 5 shows the Pitot (a) and conductivity (b) probes during the measurement process in the
 122 physical model.



123 **Figure 5 (a):** Pitot probe during the velocity measurement process. **(b):** ACM during the concentration
 124 measurement process.

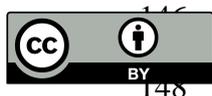
125 5. Conclusions

126 Results obtained during the tests show that aeration plays a main role in energy dissipation in
 127 open channel flows with supercritical and fully turbulent conditions. With the same water rate,
 128 higher air concentration involves lower friction head losses. This reduction has been quantified by
 129 means of the Manning roughness coefficient

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